GENERAL STRUCTURAL NOTES:

- 1.1 ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, AND THE INTERNATIONAL BUILDING CODE (IBC), 2018 EDITION.
- 1.2 DESIGN LOADING CRITERIA

THE DESIGN LOADING OF THE STRUCTURE IS AS FOLLOWS:

LIVE LOADS (IN ACCORDANCE WITH IBC TABLE 1607.1)				
OCCUPANCY OR USE	UNIFORM LIVE LOAD	CONCENTRATED LIVE LOAD	NOTES	
FLOOR, RESIDENTIAL	40-PSF	-		
BALCONIES & DECKS	60-PSF	-	1.5 x OCCUPANCY LOAD	
UNINHABITABLE ATTIC, WITH STORAGE	20-PSF	-	CONCURRENT WITH SNOW LOADS	
UNINHABITABLE ATTIC, WITHOUT STORAGE	10-PSF	-	NON-CONCURRENT WITH SNOW LOADS	
HANDRAILS AND GUARDS	-	200-LBS	ANY POINT, ANY DIRECTION (ASCE 7-16, SECTION 4.5.1)	

WIND DESIGN DATA ASCE 7-16, CHAPTER 28: SIMPLIFIED ENVELO	PE PRO	CEDURE	SEISMIC DESIGN DATA ASCE 7-16, CHAPTER 12.8: EQUIVALENT LATERAL FO	RCE PROCEDURE
BASIC DESIGN WIND SPEED (3-SEC GUST), V		100-MPH	RISK CATEGORY	II
RISK CATEGORY		II	SEISMIC IMPORTANCE FACTOR, I _e	1.0
WIND EXPOSURE		В	MAPPED SPECT ACCEL, SHORT PERIOD, Ss	1.454
INTERNAL PRESSURE COEFFICIENT		N/A	MAPPED SPECT ACCEL, 1-SEC, S ₁	0.502
EXTERIOR COMPONENTS & CLADDING		25-PSF	SITE CLASS	D
TOPOGRAPHICAL FACTOR, K _{ZT}		2.00	SPECTRAL RESPONSE COEFF, SHORT PERIOD, SDS	1.240
			SPECTRAL RESPONSE COEFF, 1-SEC, S _{D1}	0.680
SNOW LOADS			SEISMIC DESIGN CATEGORY	D
ASCE 7-16, CHAPTER 7			BASIC SEISMIC-FORCE-RESISTANCE SYSTEM	PLY. SHEAR WALLS
GROUND SNOW LOAD, Pg		25-PSF	RESPONSE MODIFICATION FACTOR, R	6.5
FLAT ROOF SNOW LOAD, P _f = 0.7 C _e C _t I _s P _g		25-PSF	SEISMIC RESPONSE COEFFICIENT, Cs	0.19
• SNOW EXPOSURE FACTOR, Ce	1.0		DESIGN BASE SHEAR, V	25.34 KIPS
• SNOW LOAD IMPORTANCE FACTOR, Is	1.0			
• THERMAL FACTOR, Ct	1.2			
DO NOT ADJUST FOR SLOPE OR DRIFT UNLESS NOTED ON THE DRAWINGS.				

SEE DRAWINGS FOR ADDITIONAL LOADING CRITERIA.

- 1.3 STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ALL OTHER PROJECT DOCUMENTS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ALL DISCREPANCIES PRIOR TO CONSTRUCTION.
- 1.4 CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE DRAWINGS.
- 1.5 CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE WORK.
- 1.6 CONTRACTOR-INITIATED CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION. CHANGES SHOWN ON SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT.
- 1.7 DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER.
- 1.8 ALL STRUCTURAL SYSTEMS COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.
- 2.1 FOUNDATION NOTES: SUBGRADE PREPARATION INCLUDING DRAINAGE, EXCAVATION, COMPACTION, AND FILLING REQUIREMENTS SHALL SHALL BEAR ON FIRM, UNDISTURBED EARTH OR CONTROLLED, COMPACTED STRUCTURAL FILL AT LEAST 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS SHOWN ON THE DRAWINGS ARE MINIMUM AND FOR GUIDANCE ONLY: THE ACTUAL ELEVATIONS SHALL BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD WORKING WITH THE TESTING LAB AND GEOTECHNICAL ENGINEER. BACKFILL BEHIND ALL RETAINING WALLS WITH FREE-DRAINING, GRANULAR FILL AND PROVIDE FOR SUBSURFACE DRAINAGE AS NOTED IN THE GEOTECHNICAL REPORT.

GEOTECHNICAL PROPERTIES AS DEFINED IN GEOTECHNICAL REPORT: AGES ENGINEERING, LLC PROJECT NO. A-1562, DATED JULY 10, 2020				
SOIL SITE CLASS	D			
ALLOWABLE SOIL BEARING PRESSURE	2500-PSF			
ACTIVE LATERAL EARTH PRESSURE (UNRESTRAINED)	35-PCF			
ACTIVE LATERAL EARTH PRESSURE (RESTRAINED)	35-PCF + 10-PSF			
SEISMIC LATERAL EARTH PRESSURE	6H-PSF			
PASSIVE LATERAL EARTH PRESSURE	325-PCF			
BASE FRICTION COEFFICIENT	0.35			

CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH IBC CHAPTER 19 AND ACI 318-14. MIX SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS. ALL CONCRETE WITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AIR-CONTENT CONFORMING TO ACI 318-14 TABLE 19.3.3.1. CONCRETE STRENGTH, BASED ON IBC SECTION 1904.1, SHALL BE AS FOLLOWS:

TYPE OR LOCATION OF CONCRETE CONSTRUCTION	MIN. 28-DAY COMPRESSIVE STRENGTH, f'c
INTERIOR SLABS-ON-GRADE	2500-PSI
FOOTINGS, BASEMENT WALLS, FOUNDATION/STEM WALLS	3000-PSI ¹
1 SPECIFIED COMPRESSIVE STRENGTH (f'a) SPECIFICATIONS ADDRESS

SERVICEABILITY REQUIREMENTS. DESIGN STRENGTH OF CONCRETE IS 2500-PSI, THEREFORE, STRENGTH TESTS ARE NOT REQUIRED. PROVIDE CONCRETE MIX TICKETS VERIFYING STRENGTH SPECIFICATIONS.

3.2 REINFORCING STEEL SHALL CONFORM TO ASTM A615/A615M-18E1 AND THE FOLLOWING:

BAR SIZE	STEEL GRADE
#5 BAR & LARGER	GRADE 60, f _y = 60,000-PSI
#4 BAR & SMALLER	GRADE 40, f _y = 40,000-PSI
WELDED WIRE FABRIC SHALL CONFORM TO	ASTM A1064/A1064M-18a

3.3 REINFORCING STEEL SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 318-14, LAP ALL CONTINUOUS REINFORCEMENT (#5 AND SMALLER) 2'-0" MINIMUM. LAPS OF LARGER BARS (#6 AND #7) SHALL BE 3'-0", MIN. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS AND LAP 2'-0" MINIMUM. LAP ADJACENT MATS OF WELDED WIRE FABRIC A MINIMUM OF 8" AT SIDES AND

NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS OTHERWISE NOTED ON THE DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.

3.4 CONCRETE PROTECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

CONDITION	CLEAR COVER
FOOTINGS & UNFORMED SURFACES CAST AGAINST & PERMANENTLY EXPOSED TO EARTH	3"
FORMED SURFACES EXPOSED TO EARTH OR WEATHER (#6 BARS & LARGER)	2"
FORMED SURFACES EXPOSED TO EARTH OR WEATHER (#5 BARS & SMALLER)	1-1/2"
SLABS & INTERIOR FACE OF WALLS (#11 BARS & SMALLER)	3/4"
COLUMN TIES, COLUMN SPIRALS, BEAM STIRRUPS	1-1/2"

6.1 FRAMING LUMBER SHALL BE KILN DRIED OR MC-19, AND GRADED AND MARKED IN CONFORMANCE WITH WCLB STANDARD GRADING RULES FOR WEST COAST LUMBER NO 17. UNLESS OTHERWISE NOTED, FURNISH TO THE FOLLOWING MINIMUM STANDARDS: Ø

MEMBER USE	SIZE	SPECIES	GRADE
STUDS	2X, 3X	HEM-FIR OR SPF	STUD
JOISTS/RAFTERS	2X, 3X	HEM-FIR	NO. 2
PLATES/MISC.	2X, 3X	HEM-FIR	NO. 2
BEAMS	4X	DOUGLAS FIR-LARCH	NO. 2
POSTS	4X	DOUGLAS FIR-LARCH	NO. 2
TIMBER BEAMS	6X & LARGER	DOUGLAS FIR-LARCH	NO. 2
TIMBER POSTS	6X & LARGER	DOUGLAS FIR-LARCH	NO. 2

6.2 GLUED LAMINATED MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH ASTM AND AITC STANDARDS. EACH MEMBER SHALL BEAR AN AITC IDENTIFICATION MARK AND SHALL BE ACCOMPANIED BY AN AITC CERTIFICATE OF CONFORMANCE. FURNISH TO THE FOLLOWING MINIMUM

MEMBER USE	COMBINATION	SPECIES	F _{bx+}	F _{bx} -	Fc⊥x	Fvx	Ex
BEAMS	24F-V4	DF/DF	2400-PSI	1850-PSI	650-PSI	265-PSI	1800-K

6.3 ENGINEERED WOOD SHOWN ON THE DRAWINGS ARE DESIGNED BASED ON TRUS JOIST ENGINEERED LUMBER MANUFACTURED BY WEYERHAEUSER IN ACCORDANCE WITH ICC REPORT NO. ES ESR-1387. ALTERNATE MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER. ALL HANGERS AND OTHER HARDWARE NOT SHOWN SHALL BE DESIGNED AND SUPPLIED BY THE JOIST MANUFACTURER. EACH PIECE SHALL BEAR A STAMP OR STAMPS NOTING THE NAME AND PLANT NUMBER OF THE MANUFACTURER, THE GRADE, THE ICC REPORT NUMBER, AND THE QUALITY CONTROL AGENCY, FURNISH TO THE FOLLOWING MINIMUM STANDARDS:

MEMBER USE	PRODUCT	F _b	F _{c⊥}	Fν	E
BEAMS	1.55E LAMINATED STRAND LUMBER (LSL)	2325-PSI	800-PSI	310-PSI	1550-KSI
BEAMS	2.0E LAMINATED VENEER LUMBER (LVL)	2600-PSI	750-PSI	285-PSI	2000-KSI
BEAMS	2.2E PARALLEL STRAND LUMBER (PSL)	2900-PSI	750-PSI	290-PSI	2200-KSI
RIM BOARDS	LAMINATED STRAND LUMBER (LSL)	1700-PSI	680-PSI	400-PSI	1300-KSI

- 6.4 ENGINEERED WOOD I-JOISTS SHOWN ON THE DRAWINGS ARE DESIGNED BASED ON TRUS JOIST I-JOISTS MANUFACTURED BY WEYERHAEUSER IN ACCORDANCE WITH ICC REPORT NO. ES ESR-1153. ALTERNATE ENGINEERED WOOD I-JOISTS MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER.
- 6.5 PREFABRICATED CONNECTOR PLATE WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH TPI 1-2014 FOR THE SPANS AND CONDITIONS SHOWN ON THE DRAWINGS. WOOD TRUSSES SHALL UTILIZE APPROVED CONNECTOR PLATES (MITEK, ITW OR OTHER APPROVED TRUSS PLATE MANUFACTURER).

UNLESS OTHERWISE NOTED, LOADING SHALL BE AS FOLLOWS:

ROOF TRUSS DESIGN LOADING		FLOOR TRUSS DESIGN LOADING		
MEMBER/USE	UNIFORM LOAD	MEMBER/USE	UNIFORM LOAD	
TOP CHORD SNOW LOAD	25-PSF	TOP CHORD LIVE LOAD	40-PSF	
TOP CHORD WIND LOAD (UPLIFT)	15-PSF	TOP CHORD DEAD LOAD	10-PSF	
TOP CHORD DEAD LOAD	7-PSF			
BOTTOM CHORD LIVE LOAD	10-PSF	BOTTOM CHORD LIVE LOAD	N/A	
BOTTOM CHORD DEAD LOAD	5-PSF	BOTTOM CHORD DEAD LOAD	5-PSF	

SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS PRIOR TO FABRICATION. SUBMITTED DOCUMENTS SHALL BEAR THE STAMP AND SIGNATURE OF A REGISTERED PROFESSIONAL ENGINEER, STATE OF WASHINGTON. TRUSS DESIGN DRAWINGS SHALL INCLUDE, AT A MINIMUM, THE FOLLOWING:

- 1. SLOPE OR DEPTH, SPAN AND SPACING
- LOCATION OF ALL JOINTS AND SUPPORT LOCATIONS NUMBER OF PLIES IF GREATER THAN ONE
- REQUIRED BEARING WIDTHS
- DESIGN LOADS AND LOCATIONS: INCLUDE TOP AND BOTTOM CHORD LIVE AND DEAD LOADS, GIRDER LOADS, AND ENVIRONMENTAL LOADS (SEISMIC, WIND, SNOW, ETC.)
- 6. OTHER LATERAL LOADS, INCLUDING DRAG STRUT LOADS
- ADJUSTMENTS TO WOOD AND METAL CONNECTOR PLATE DESIGN VALUE FOR CONDITIONS OF USE
- MAXIMUM REACTION FORCE AND DIRECTION (INCLUDING MAXIMUM UPLIFT)
- 9. METAL-CONNECTOR-PLATE TYPE, SIZE, THICKNESS, AND LOCATION

13. MAXIMUM AXIAL TENSION AND COMPRESSION FORCES IN EACH TRUSS MEMBER

- 10. SIZE SPECIES AND GRADE FOR EACH MEMBER
- 11. TRUSS-TO-TRUSS CONNECTIONS AND TRUSS FIELD ASSEMBLY REQUIREMENTS 12. CALCULATED SPAN-TO-DEFLECTION RATIO AND MAXIMUM VERTICAL AND HORIZONTAL DEFLECTION FOR LIVE AND TOTAL LOADS
- 14. REQUIRED PERMANENT INDIVIDUAL TRUSS MEMBER RESTRAINT LOCATION AND THE METHOD AND DETAILS OF RESTRAINT BRACING TO
- 15. PLACEMENT LAYOUT INCLUDING BEARING POINTS, INTERSECTIONS, HIPS, VALLEYS, ETC. 16. TRUSS-TO-TRUSS AND TRUSS-TO-BEAM CONNECTION DETAILS AND HARDWARE
- 6.6 ROOF, FLOOR & WALL SHEATHING SHALL BE APA RATED, EXTERIOR OR EXPOSURE 1 PLYWOOD OR OSB MANUFACTURED UNDER THE PROVISIONS OF VOLUNTARY PRODUCT STANDARDS DOC PS-1 OR DOC PS-2, OR APA PRP-108 PERFORMANCE STANDARDS AND POLICIES FOR STRUCTURAL USE PANELS. SEE DRAWINGS FOR THICKNESS, SPAN RATING, AND NAILING REQUIREMENTS. UNLESS OTHERWISE NOTED, WALL SHEATHING SHALL BE 1/2" (NOMINAL) WITH SPAN RATING OF 24/0. GLUE FLOOR SHEATHING TO ALL SUPPORTING MEMBERS WITH ADHESIVE CONFORMING TO APA SPECIFICATION AFG-01.
- 6.7 WOOD MEMBERS SHALL BE PROTECTED AGAINST DECAY AND TERMITES IN ACCORDANCE WITH IBC SECTION 2304.12. WHERE REQUIRED, MEMBERS SHALL BE NATURALLY DURABLE SPECIES OR SHALL BE TREATED WITH WATERBORNE PRESERVATIVES WOOD IN ACCORDANCE WITH AMERICAN WOOD PROTECTION ASSOCIATION SPECIFICATION AWPA U1. MEMBERS SHALL BE CLEARLY LABELED. MODIFED TREATED MEMBERS (RIPPED OR END CUT) SHALL BE FIELD TREATED IN ACCORDANCE WITH SPECIFICATION AWPA M4.
- 6.8 TIMBER CONNECTORS AND PROPRIETARY FASTENERS SHALL BE "STRONG-TIE" BY SIMPSON COMPANY, AS SPECIFIED IN THEIR CURRENT CATALOG. PROVIDE NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER. CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. WHERE CONNECTOR STRAPS CONNECT TWO MEMBERS, CENTER STRAP ON JOINT AND PROVIDE NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER, WITH EQUAL NUMBER AND SIZE OF FASTENERS IN EACH MEMBER.

ALTERNATE HARDWARE MANUFACTURER SUBSTITUTIONS, SUCH AS USP CONNECTORS, SHALL BE ICC APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. ALL JOIST HANGERS AND OTHER HARDWARE SHALL BE COMPATIBLE IN SIZE WITH SPECIFIED FRAMING MEMBERS. SEE HANGER CONVERSION TABLE FOR PRE-APPROVED SUBSTITUTIONS.

TIMBER CONNECTORS AND THEIR FASTENERS SHALL BE PROTECTED FROM CORROSION IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS OR ASTM A 653, TYPE G185.

6.9 DOWEL-TYPE FASTENERS (BOLTS, LAG SCREWS, WOOD SCREWS AND NAILS) SHALL CONFORM TO SECTIONS 11 & 12 OF THE ANSI/AWC

DOWEL TYPE FASTENER	GRADE	REQUIREMENTS AT EXTERIOR USE OR WHEN IN CONTACT w/ TREATED LUMBER	INSTALLATION
BOLTS	ASTM A307	ASTM B695, CLASS 55 GALVANIZED or STAINLESS STEEL	ANSI/AWC NDS-2018 SECTION 12.1.3 HOLE = BOLT Ø + (1/32" to 1/16") WASHER @ BOLT HEAD & @ NUT
ALL-THREAD/THREADED ROD	ASTM F1554	ASTM B695, CLASS 55 GALVANIZED or STAINLESS STEEL	ANSI/AWC NDS-2018 SECTION 12.1.3 HOLE = BOLT Ø + (1/32" to 1/16") WASHER @ BOLT HEAD & @ NUT
LAG SCREWS	ASTM A307	ASTM A153 GALVANIZED or STAINLESS STEEL	ANSI/AWC NDS-2018 SECTION 12.1.4 LEAD HOLE = 0.5 SHANK Ø; SHANK HOLE = SHANK Ø WASHER @ LAG HEAD
WOOD SCREWS		ASTM A153 GALVANIZED or STAINLESS STEEL	ANSI/AWC NDS-2018 SECTION 12.1.5 PILOT HOLE = 0.5 ROOT Ø (UNLESS SELF- BORING)
NAILS	ASTM F1667	ASTM A153 GALVANIZED or STAINLESS STEEL	ANSI/AWC NDS-2018 SECTION 12.1.6 AVOID OVERDRIVING or UNDERDRIVING AVOID WOOD SPLITTING TOENAILS 30°, 1/3 NAIL LENGTH FROM JOINT

NAILS SPECIFIED ON DRAWINGS SHALL BE AS FOLLOWS, UNLESS OTHERWISE NOTED:

IAIL USE	PENNY WEIGHT	SIZE
RAMING NAILS	12d BOX	0.131"Ø x 3-1/4"
HEATHING NAILS	8d BOX	0.131"Ø x 2-1/2"

ALL METAL FASTENERS EXPOSED TO WEATHER OR IN CONTACT WITH TREATED WOOD SHALL BE PROTECTED FROM CORROSION ACCORDING TO TABLE ABOVE. NUTS AND BOLTS EXPOSED TO WEATHER OR IN CONTACT WITH TREATED WOOD SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM A153/A153M-16a OR STAINLESS STEEL. SEE ABOVE FOR PROPRIETARY FASTENER REQUIREMENTS. DO NOT SUBSTITUTE STANDARD DOWEL-TYPE FASTENERS FOR PROPRIETARY FASTENERS UNLESS SPECIFICALLY ALLOWED.

7.1 SPECIAL INSPECTION IN ACCORDANCE WITH IBC SECTION 1704.2 IS NOT REQUIRED. STANDARD INSPECTIONS SHALL BE IN ACCORDANCE WITH IBC SECTION 110.

7.2 SPECIAL INSPECTION IN ACCORDANCE WITH IBC SECTION 1704.2 IS REQUIRED IN ACCORDANCE WITH STATEMENT OF SPECIAL INSPECTIONS.

STATEMENT OF SPECIAL INSPECTION SPECIAL INSPECTION & TESTING REQUIREMENTS						
	SOILS	-	-	REFER TO GEOTECHNICAL REPORT		
	CONCRETE	-	-	PROVIDE BATCH MIX TICKETS		
	CONCRETE RETAINING WALLS	-	Х			
	POST-INSTALLED ANCHORS IN CONCRETE	Х	-			

STANDARD INSPECTIONS SHALL BE IN ACCORDANCE WITH IBC SECTION 110.

- 7.3 STRUCTURAL OBSERVATION IN ACCORDANCE WITH IBC SECTION 1704.6 IS REQUIRED. CONTACT STRUCTURAL ENGINEER ONE WEEK PRIOR TO COORDINATE SITE VISIT.
- 7.4 STRUCTURAL OBSERVATION IN ACCORDANCE WITH IBC SECTION 1704.6 IS NOT REQUIRED.

24-010-02

BTL

PROJECT NUMBER: PROJECT MANAGER: PROJECT ENGINEER:

REVISIONS:

DRAWN BY:

NO.	DESCRIPTION	DATE
	PERMIT	03/12/2024

19011 Woodinville-Snohomish Road NE, Suite 100 Woodinville, WA 98072-4436 Phone: 425-814-8448 Fax: 425-821-2120 ARCHITECTS 10801 Main Street, #110 Bellevue, WA 98004

ENGINEERING

GENERAL STRUCTURAL NOTES

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24-010-02

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ARCHITECTS

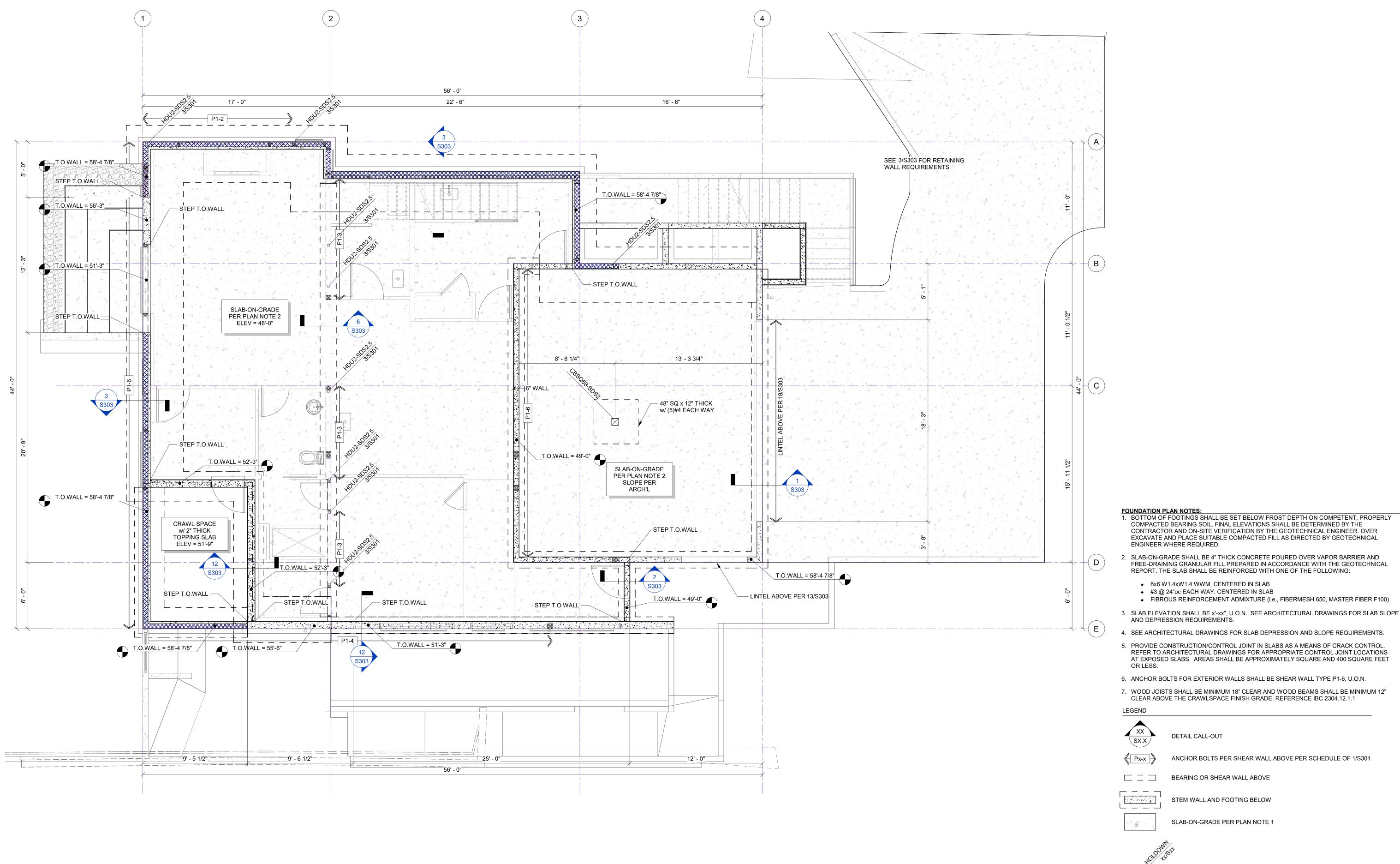
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FOUNDATION PLAN

1 2 4 8 16

HOLDOWN TO WALL ABOVE



1/4" = 1'-0"

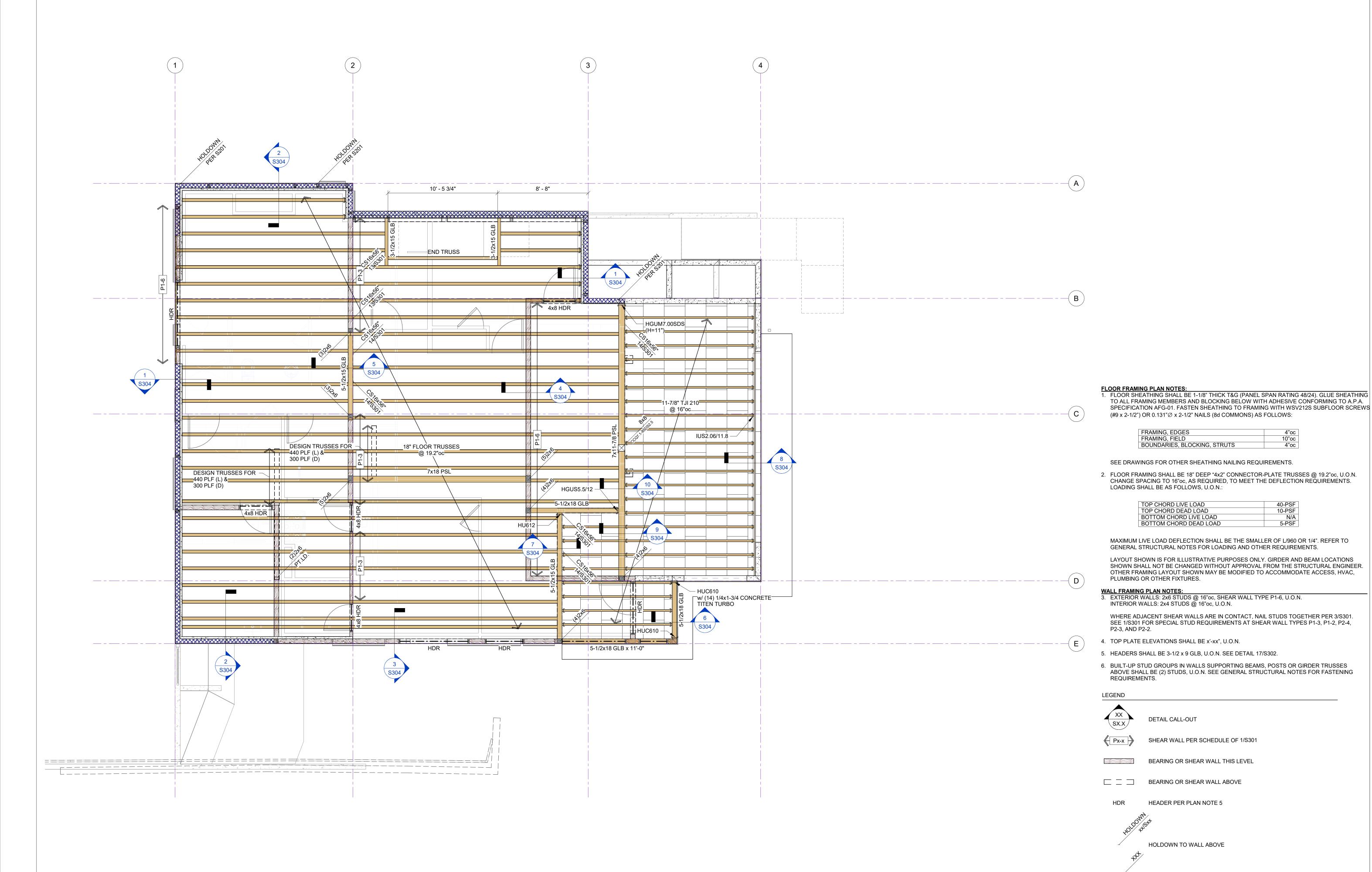
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MAIN FLOOR FRAMING PLAN

POST BELOW ((2)2x u.o.n.)



1 MAIN FLOOR FRAMING 1/4" = 1'-0"

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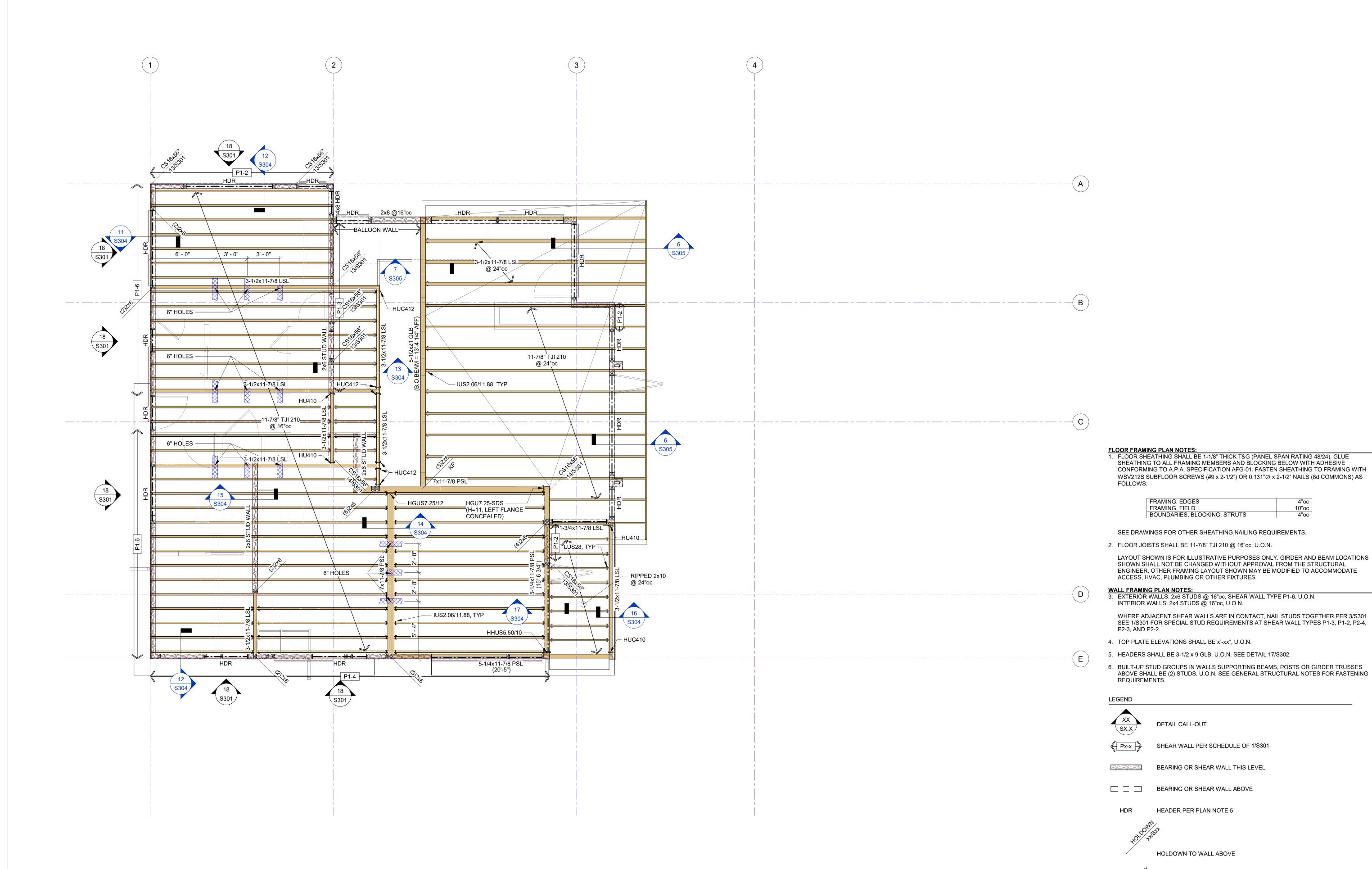
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UPPER FLOOR FRAMING PLAN

1 UPPER FLOOR FRAMING 1/4" = 1'-0"

POST BELOW



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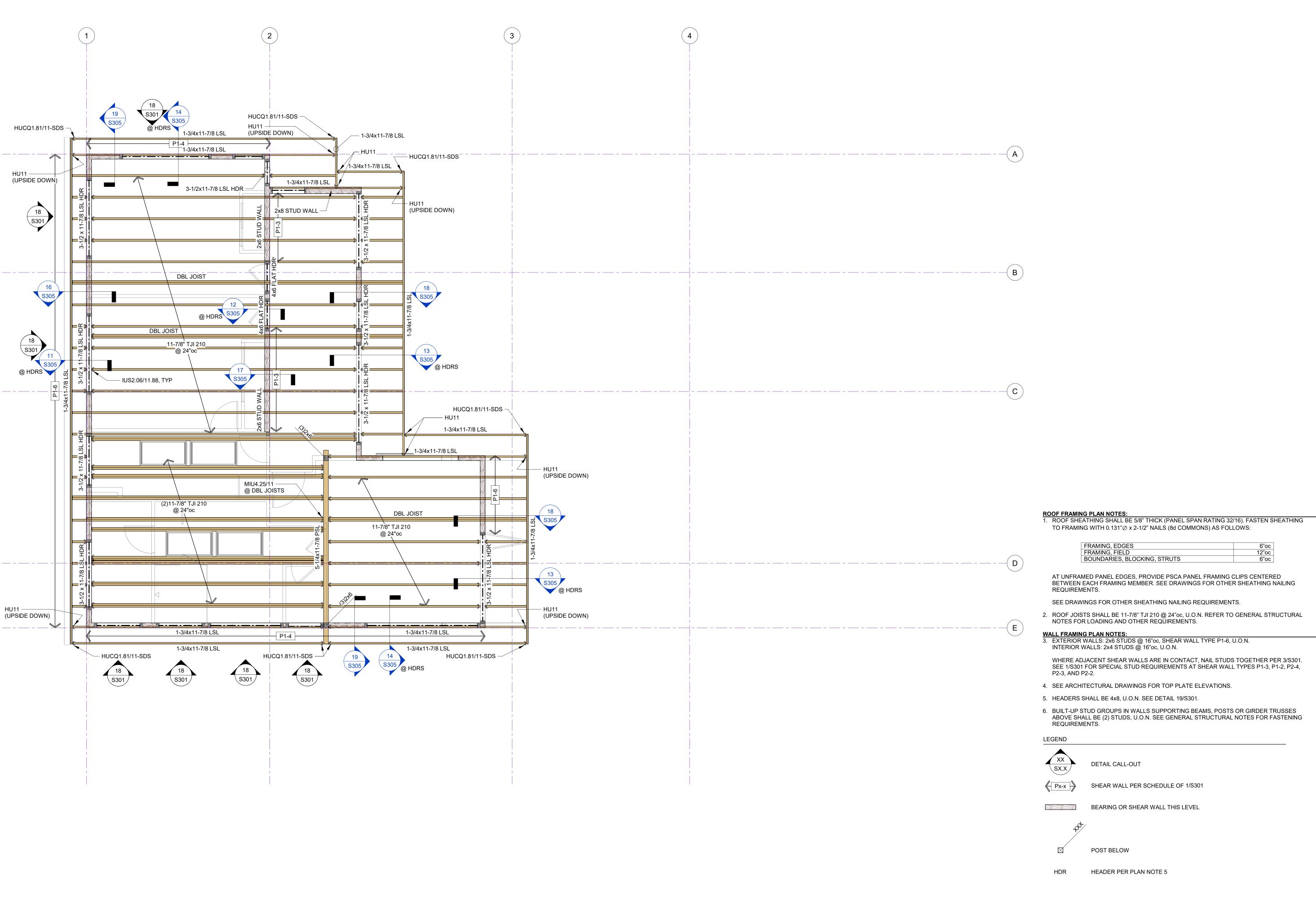
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ROOF FRAMING PLAN





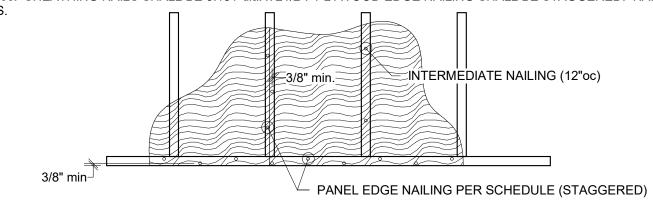
SHEAR WALL SCHEDULE

(IN ACCORDANCE w/ ANSI/AF&PA SDPWS-2015 SECTION 4.3)

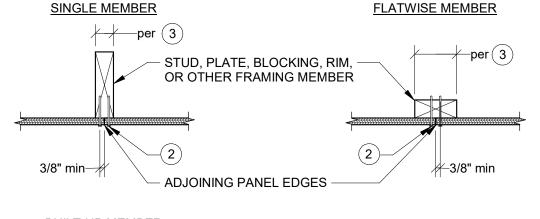
						UPDATED 4/2	0/2021				
WALL TYPE	SHEATHING 1	PANEL EDGE NAILING	MINIMUM WIDTH OF NAILED FACE OF FRAMING @ ADJOINING PANEL EDGES		MUDSILL PLATE	FACE NAILING	FRAMING CLIPS	ANCHORAGE TO CONCRETE 6		SEISMIC CAPACITY h/b = 2	WIND CAPACITY h/b = 2
			SINGLE MEMBER	BUILT-UP MEMBER				ANCHOR BOLTS	MUDSILL ANCHORS	(h/b = 3.5)	(h/b = 3.5)
P1-6	1 Side	6"oc	2x	-	2x	6"oc	A35 @ 27"oc or LTP4 @ 27"oc	5/8"Ø @ 60"oc	MASAP @ 52"oc	240-plf (194-plf)	240-plf (194-plf)
P1-4	1 Side	4"oc	2x	-	2x	4"oc	A35 @ 18"oc or LTP4 @ 18"oc	5/8"Ø @ 46"oc	MASAP @ 36"oc	350-plf (284-plf)	350-plf (284-plf)
P1-3	1 Side	3"oc	3x	(2)2x	2x	3"oc	A35 @ 14"oc or LTP4 @ 14"oc	5/8"Ø @ 36"oc	MASAP @ 28"oc	450-plf (366-plf)	450-plf (366-plf)
P1-2	2 Side	2"oc	3x	(2)2x	2x	2"oc	A35 @ 7-1/2"oc or LTP4 @ 7-1/2"oc	5/8"Ø @ 20"oc	MASAP @ 18"oc	590-plf (478-plf)	820-plf (669-plf)
P2-4	2 Side	4"oc	3x	(2)2x	2x	4"oc	A35 @ 18"oc <u>AND</u> LTP4 @ 18"oc	5/8"Ø @ 28"oc	MASAP @ 15"oc	700-plf (568-plf)	700-plf (568-plf)
P2-3	2 Side	3"oc	3x	(2)2x	2x	3"oc	A35 @ 14"oc <u>AND</u> LTP4 @ 14"oc	5/8"Ø @ 22"oc	MASAP @ 11"oc	900-plf (733-plf)	900-plf (733-plf)
P2-2	2 Side	2"oc	3x	(2)2x	2x	2"oc	A35 @ 8"oc <u>AND</u> LTP4 @ 8"oc	5/8"Ø @ 12"oc	MASAP @ 7"oc	1180-plf (957-plf)	1640-plf (1338-plf)

SHEAR WALL SCHEDULE NOTES

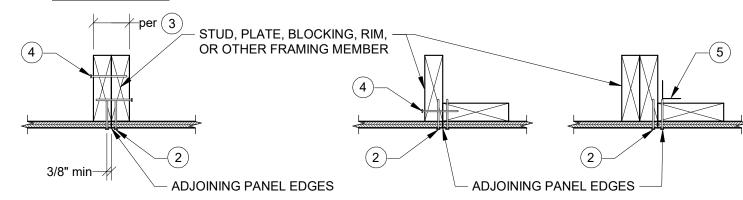
- (SECTION 4.3.7.1.1)
 7/16" OSB or 15/32" PLYWOOD SHEATHING OR SIDING EXCEPT GROUP 5 SPECIES. MINIMUM PANEL SPAN RATING OF (24/0). PANELS SHALL NOT BE LESS THAN 4'x8', EXCEPT AT BOUNDARIES AND CHANGES IN FRAMING. ALL EDGES OF ALL PANELS SHALL BE SUPPORTED BY AND FASTENED TO FRAMING MEMBERS OR BLOCKING.
- (SECTION 4.3.7.1.2. & SECTION 4.3.7.1.3) PANEL EDGE NAILING APPLIES TO ALL SHEATHING PANEL EDGES. NAIL SHEATHING TO INTERMEDIATE FRAMING MEMBERS WITH SHEATHING NAILS @ 12"oc. MAXIMUM STUD SPACING SHALL BE 16"oc. SHEATHING NAILS SHALL BE 0.131"dia. x 21/2". PLYWOOD EDGE NAILING SHALL BE STAGGERED. NAILS SHALL BE LOCATED AT LEAST 3/8" FROM THE PANEL EDGES.



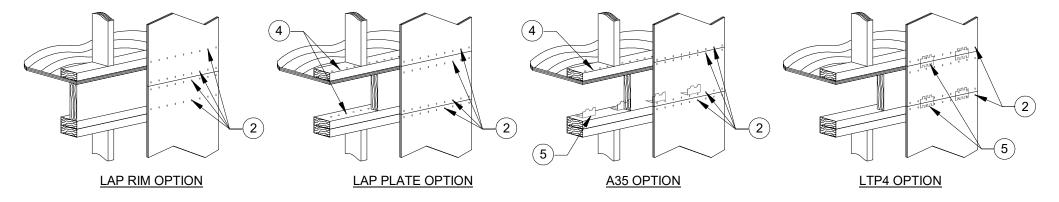
3 (SECTION 4.3.7.1.4) THE MINIMUM NOMINAL WIDTH OF THE NAILED FACE OF FRAMING AND BLOCKING AT ADJOINING PANEL EDGES SHALL BE AS INDICATED IN THE SCHEDULE.



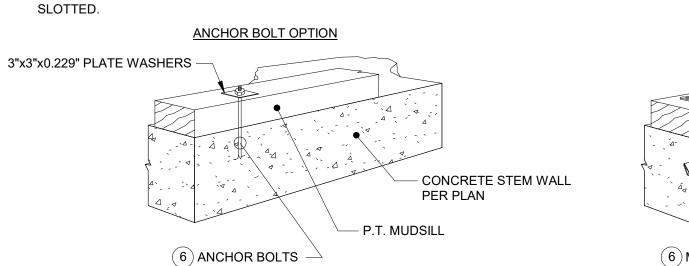
BUILT-UP MEMBER

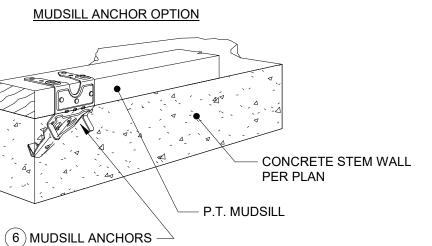


- FACE NAILING APPLIES TO CONDITIONS WHERE FRAMING NAILS CAN BE STRAIGHT DRIVEN THRU FIRST MEMBER AND PENETRATE MAIN MEMBER MINIMUM OF 1-1/2". FRAMING NAILS SHALL BE 0.131"dia. x 3-1/4". 0.131"dia. x 3" NAILS MAY BE USED WHEN STITCHING TOGETHER (2)2x MEMBERS WITH NO SPACERS.
- AT ADJOINING PANEL EDGES WHERE SHEATHING CANNOT LAP ON SINGLE MEMBER AND FACE NAILING CANNOT BE ACCOMPLISHED, FRAMING CLIPS SHALL BE USED TO FASTEN BUILT-UP MEMBERS. USE 0.131"dia. x 2-1/2" NAILS AT LTP4 CLIP WHEN INSTALLED OVER 1/2" SHEATHING.



(6) (SECTION 4.3.6.4.3) ANCHOR BOLTS EMBEDMENT SHALL BE 7", U.O.N. ALL ANCHORS SHALL HAVE 3" x 3" x 0.229" PLATE WASHERS. PLATE WASHER SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE BOTTOM PLATE ON THE SIDE WITH SHEATHING. IF SHEATHING IS ON BOTH SIDES OF THE WALL, STAGGER THE ANCHOR BOLTS, AS REQUIRED, SO THAT HALF OF THE PLATE WASHERS ARE WITHIN 1/2" OF THE EDGE OF THE BOTTOM PLATE ON EACH SIDE. HOLE IN PLATE WASHERS MAY BE DIAGONALLY





/12/2024

PROJECT NUMBER: 24-010-02 PROJECT MANAGER: BTL PROJECT ENGINEER: DRAWN BY:

REVISIONS:

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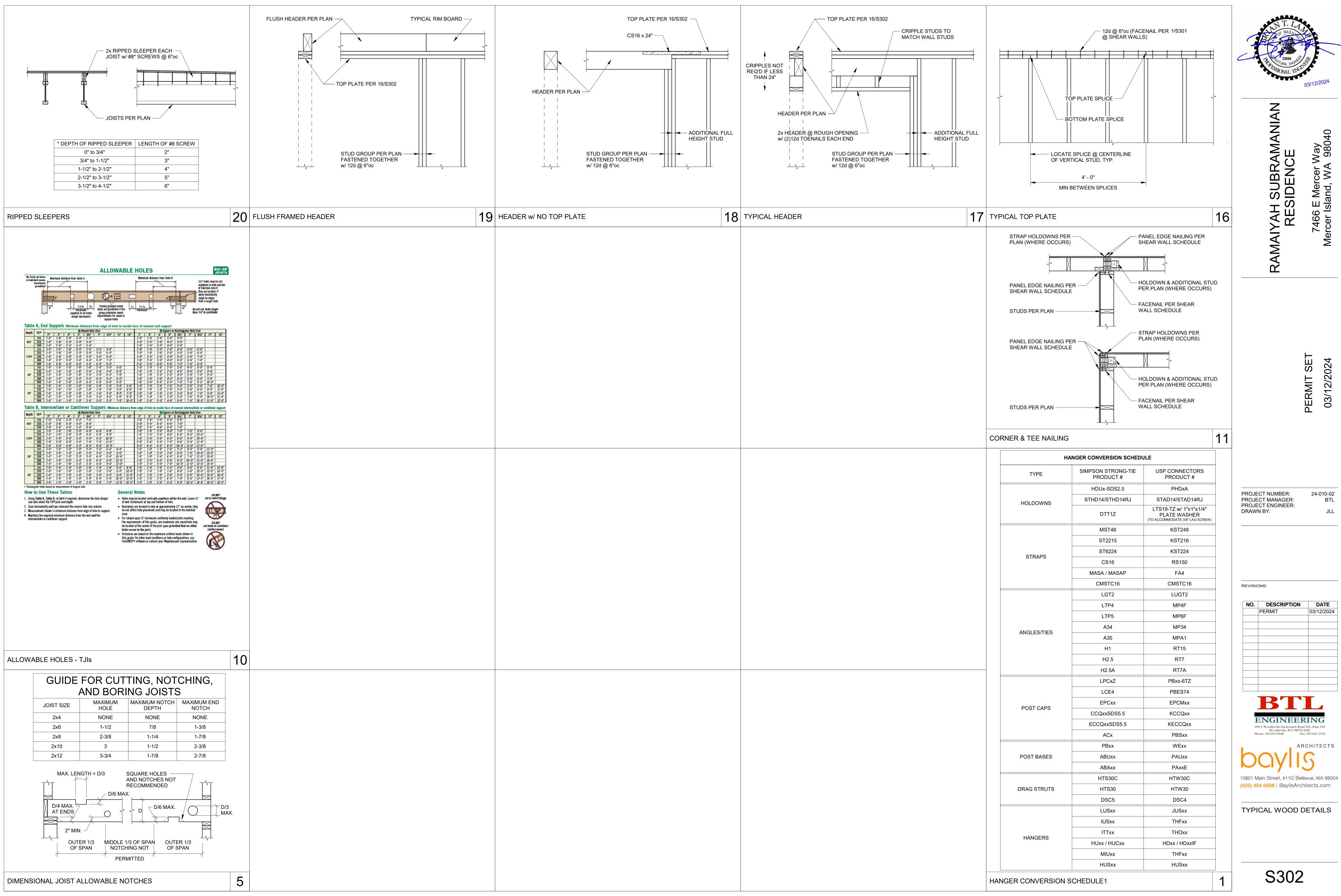


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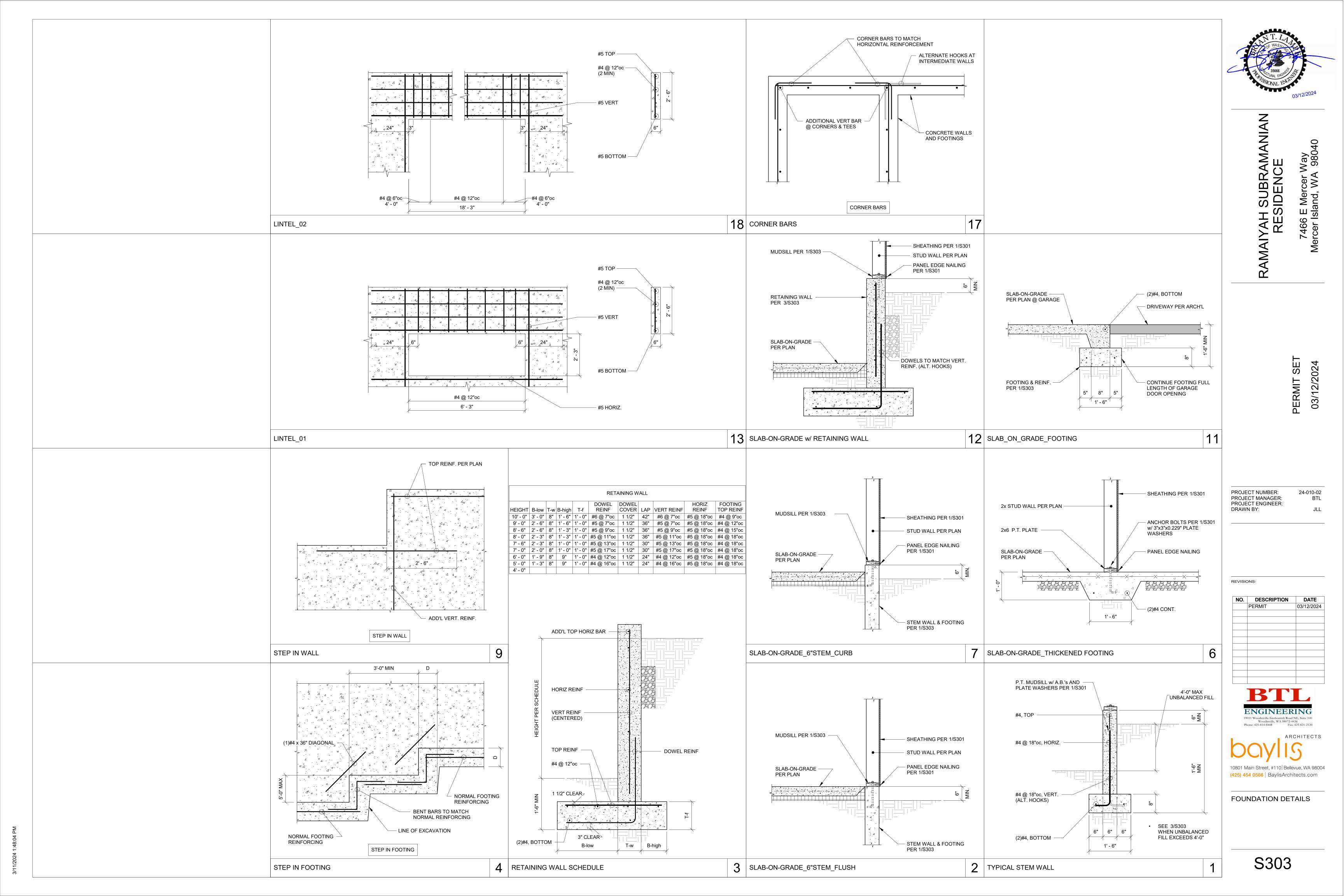
SHEAR WALL DETAILS

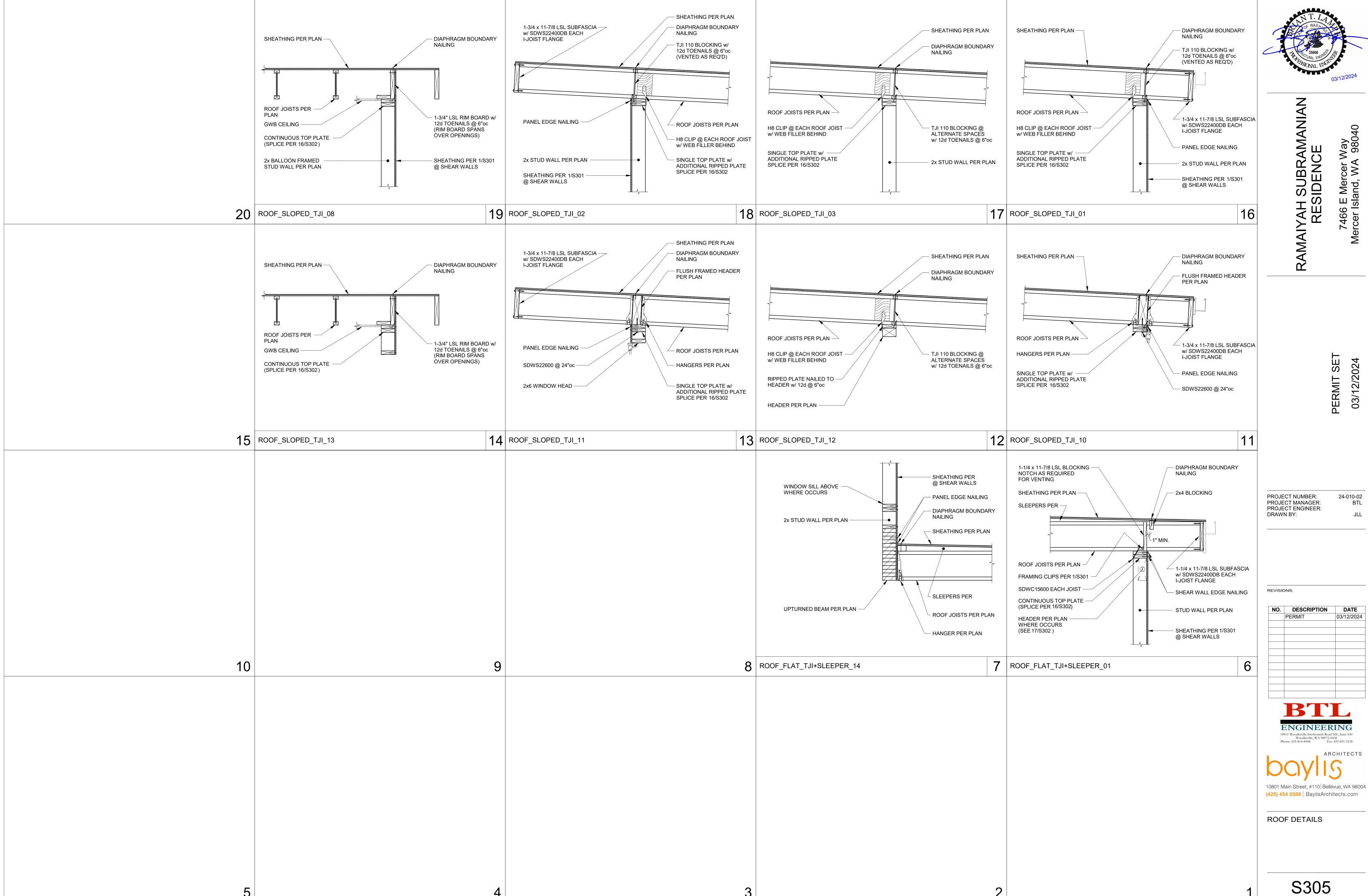
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DATE 03/12/2024 ENGINEERING

ARCHITECTS





NO.	DESCRIPTION	DATE
	PERMIT	03/12/2024
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